

PORTLAND CEMENT CONCRETE FIELD SECTION 501

501.1 SCOPE. To establish uniformity in mix design and inspection of portland cement concrete. The instructions contained in this Section are intended to supplement those contained in the Construction Manua Sec 500.

501.2 PROCEDURE.

501.2.1 Material Inspection. Each material to be incorporated into a concrete mix is to be inspected and reported in accordance with procedures found in the corresponding section of this Manual. Materials incorporated into commercial mix concrete need not be reported. Calibration of the plant and performance of uniformity tests of concrete are to be performed in accordance with General Sec 10 of this Manual.

501.2.2 Mix Design. The district Operations Engineer is responsible for furnishing concrete mix designs to the Resident Engineer. The mix design should be furnished as soon as the contractor's material sources are known or when requested by Construction. Mix design information should be furnished, by letter, to the Resident Engineer, from the district Operations Engineer. An example of one type of letter is shown as Exhibit 504A of this Section. In the following paragraphs are two methods of designing concrete mixes. Method A is to be used for designing portland cement concrete pavement mixes and may be used to design masonry concrete mixes. Method B may be used for designing masonry concrete mixes but may not be used for designing pavement mixes. Mix design proportions are to be rounded to the nearest 0.05 except for low slump and latex modified mixes which are to be rounded to the nearest 0.01.

Note: Some mix design factors are controlled by the specifications. Others are chosen by district personnel. A suggested percent of fine aggregate is 38% for all pavement mixes. For masonry mixes, it is suggested that 36% to 38% fine aggregate be used for air entrained mixes and 40% for nonair entrained mixes. Another suggested design factor is to use a minimum of 6.08sacks of cement per cubic yard in paving mixes.

501.2.2.1 Method A - Concrete Mix Design by Absolute Volumes.

(a) Design assumptions.

Design a PCCP mix using 15% fly ash, one size coarse aggregate and the following mix parameters:

Cement factor = 6.08 sacks/cu yd

Percent fine aggregate by absolute volume = 38

Water content = 4.9 gal/sack*

Percent air content = 5.5

*Note: This value is chosen well below the specified maximum to allow for expected fluctuations in the mix characteristics during production. When a waterreducing admixture is tobe used, the assumed water content would be reduced according to manufacturer's recommendations.

(b) Physical characteristics. Weight per cubic foot, specific gravity, and percent absorption for the aggregates are obtained from "Physical Characteristics of Principal Portland Cement Concrete Aggregates in Missouri" or if sources are new, from test results of samples submitted for rodded unit weight, specific gravity and absorption as received as required in Field Sec 1001 of this Manual. Absolute volume may be obtained from Table 1 of this Section or calculated as follows:



Absolute volume =	Dry rodded unit weight (lb/cu ft)
	(Specific gravity) x (62.4)

For the example use:				
•	Unit Weight (lb/cu ft	Specific Gravity	Percent Absorption	Abs Vol (cu.ft.)
Fine aggregate -				
Class A, MO. River	111	2.61	0.4	0.6816
Coarse aggregate - Burlington Limestone	96	2.62	1.0	0.5872
Cement (constant values)	94	3.15		0.4782
Fly Ash -				

^{*}Assumed to be equivalent to the unit weight of cement for design purposes.

94*

(c) Design example. Mix proportions are established on a onesack mix. To determine the absolute volume of the aggregates in a onesack batch, it is necessary to first determine the total absolute volume of the concrete and the absolute volume of entrained air, cement, and water.

0.5538

2.72

Absolute volumes (cu ft) per sack of cement:

MO. Portland, Louisa

Absolute volume Total concrete	=	27.0 cu ft/cu yd 6.08 sacks/cu yd
	=	4.4408 cu ft/sack
Absolute volume Entrained air	=	(Total absolute volume of concrete) x (Percent Air) 100
	=	(4.4408) (5.5) 100
	=	0.2442 cu ft/sack
Absolute Volume Cement	=	(0.85) (0.4782)
Cement	=	0.4065 cu ft/sack
Absolute Volume Fly Ash	=	(0.15) (0.5538)
TTy Asii	=	0.0831 cu ft/sack
Absolute Volume Water	=	4.9 gal/sack 7.5 gal/cu ft
	=	0.6533 cu ft/sack

(Total abs. vol conc.) - (Abs. vol. of air, cement, flyash, and water)



Absolute Volume

total aggregates

$$= (4.4408) - (0.2442 + 0.4065 + 0.0831 + 0.6533)$$

= 3.0537 cu ft/sack

Absolute volume Fine aggregates

(Total aggr. abs. vol.)(Design percent fine aggr.)
100

= <u>(3.0537)(38)</u> 100

= 1.1604 cu ft/sack

Mix proportions of aggregates are equal to the absolute volume of that aggregate in a one-sack batch divided by the absolute volume of one cubic foot (dry rodded) of that aggregate.

Fine aggregate = $\frac{1.1604}{0.6816}$ proportion = $\frac{1.7025}{0.6816}$

The mix proportion for fine aggregate, rounded to the nearest 0.05, would be 1.70.

Absolute volume = (Total aggr. abs. vol.)- (Fine aggr. abs. vol.)

Coarse aggregate

 $= (3.0537) - (1.70 \times 0.6816)$

= 1.8950 cu ft/sack

Coarse aggregate = $\frac{1.8950}{0.5872}$

= 3.2272

Therefore, mix proportions rounded to the nearest 0.05, of cement, fine aggregate, and coarse aggregate are:

1: 1.70: 3.25

Mix proportions are defined as the volume of dry-rodded fine aggregate (1.70 cu ft) and coarse aggregate (3.25 cu ft) required for a mix containing one sack (1.0 cu ft) of cementitious material (cement + fly ash). After mix proportions have been established, the mix characteristics, including total yield for a one cubic yard batch, shall be calculated as shown in paragraphs 501.2.3 through 501.2.3.5 of this Section.

501.2.2.2 Method B - Concrete Mix Design for any Cement Factor and any Percent Excess Mortar.

(a) Design assumptions.

Design a B-1 mix using 15% fly ash and the following mix parameters:



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Cement factor = 6.60 sacks/cu yd

Percent desired excess mortar = 90 Percent air content = 5.5

(b) Physical characteristics. Weight per cubic foot, specific gravity, and percent absorption for the aggregate are obtained from "Physical Characteristics of Principal Portland Cement Concrete Aggregate in Missouri" or if sources are new, from test results of samples submitted for rodded unit weight, specific gravity, and absorption-as-received, as required in Field Sec 1001 of this Manual. Absolute volume may be obtained from Table 1 of this Section or calculated as follows:

Absolute volume = <u>Dry rodded unit weight (lbs/cu ft)</u> (Specific gravity) x (62.4)

For the example use,

7	Jnit Weight	Specific Gravity	Percent Absorption	Abs Vol (cu ft)
Fine aggregate -	b/cu.ft)			
Class A, Mo.River	111	2.61	0.4	0.6816
Coarse aggregate - Burlington Limestone	96	2.62	1.0	0.5872
Cement (constant values)	94	3.15		0.4782
Fly Ash - Mo. Portland,Louisa	94*	2.72		0.5538

^{*}Assumed to be equivalent to the unit weight of cement for design purposes.

(c) Design example. The mix proportions are established on a onesack mix. This method of design requires that the coarse aggregate content be solved first. The cement, fly ash, air, and water content are known from the assumptions for design. Let "A" equal the cubic feet of coarse aggregate (dry rodded) required for a one sack batch.

Absolute volumes (cu ft) per sack of cement:

Absolute volume
Total concrete

= \frac{27.0 \text{ cu ft/cu yd}}{6.60 \text{ sacks/cu yd}}

= \frac{4.0909 \text{ cu ft/sack}}{4.0909 \text{ cu ft/sack}}

Absolute volume
Coarse aggregate = \frac{(Coarse aggregate content)}{(Absolute volume of 1 \text{ cu ft}}

dry rodded aggregate)

= \frac{0.5872 (A) \text{ cu ft/sack}}{4.0909} = \frac{(Absolute volume of 1 \text{ cu ft})}{4.0909} = \frac{(Absolute volume of 1 \text{ cu ft/sack}}{4.0909} = \frac{(Absolute volume of 1 \text



Since the design is assuming 90 percent more mortar than is necessary to fill the voids in the coarse aggregate, solve for the cubic feet of voids in the coarse aggregate and multiply by 1.90 to determine absolute volume of mortar.

Voids in coarse aggregate = (Coarse aggr content) - (Abs vol of coarse aggr)

= (A) - (0.5872) (A)

= 0.4128 (A) cu ft/sack

Absolute volume of mortar = (0.4128) (A) x (1.90)

= 0.7843 (A) cu ft/sack (Equation 2)

The absolute volume of mortar is set to two equations, both containing the unknown coarse aggregate content "A". Solve the two equations for coarse aggregate as follows:

0.7843 (A) = 4.0909 - 0.5872 (A) 1.3715 (A) = 4.0909 A = 2.9828 cu ft/sack Rounded to the nearest 0.05, use 3.00 cu ft/sack

The method of calculation of the coarse aggregate content shown in this paragraph could have been summarized in the following formula:

A = $\frac{27}{\text{C.F.}}$ $(1 - \text{VCA}) (\frac{100 + \text{E.M.}}{100}) + \text{VCA}$

Where:

A = Coarse aggregate content C.F. = Cement factor, sacks E.M. = Percent excess mortar

VCA = Absolute volume of one cubic foot of coarse aggregate.

To determine the absolute volume of fine aggregate in a one-sack batch, deduct the sum of absolute volumes of coarse aggregate, cement, fly ash, air and water from the total absolute volume of concrete per one-sack batch.

Absolute = (Total absolute volume concrete) - (Absolute volume coarse aggregate + cement + fly ash+air + water) volume

fine aggregate

Absolute volumes (cu ft) per sack of cement:

Absolute volume = (Coarse aggregate content) x (Absolute volume of 1 cu ft)

coarse aggregate = (3.00) x (0.5872)

= 1.7616 cu ft/sack



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Absolute volume (0.85)(0.4782)

cement

0.4065 cu ft/sack

Absolute volume (0.15)(0.5538)

0.0831 cu ft/sack

fly ash

Absolute volume (Total absolute volume of concrete) (Percent air) 100

entrained air

(4.0909) x (5.5) 100

0.2250 cu ft/sack

Absolute volume 5.0 gal/sack water 7.5 gal/cu ft

0.6667 cu ft/sack

Absolute volume (Total abs vol conc) - (Abs vol coarse aggr) + cement + fly ash+ air + water)

fine aggregate

(4.0909) - (1.7616 + 0.4065 + 0.0831 + 0.2250 + 0.6667)

0.9480 cu ft/sack

Volume of dry rodded = Abs. vol. of fine aggregate

fine aggregate Abs. vol. of 1 cu. ft. of dry rodded aggr.

> 0.9480 0.6816

1.3908 cy ft/sack

Rounded to the nearest 0.05, use 1.40 cu.ft./sack.

Therefore, mix proportions, rounded to the nearest 0.05, of cement, fine aggregate and coarse aggregate are:

1: 1.40: 3.00

After mix proportions have been established, the mix characteristics, including total yield for a one cubic yard batch, shall be calculated as shown in paragraphs 501.2.3 through 501.2.3.5 of this Section.

501.2.3 Mix Characteristics. The characteristics of the concrete mixture shall be calculated to verify that the mix proportions will result in a mix having the characteristics assumed for design. Characteristics ofdesign mixtures should be furnished to the Resident Engineer with the mix proportions. After the proportions are established by either Method A or B, the calculations for mix characteristics are the same. For the purpose of this example, the proportions and materials from Method A will be used.



501.2.3.1 Cement Factor. The cement factor is defined as the cement content in sacks per cubic yard of concrete, based on the absolute volumes of all the ingedients, including entrained air when specified. By arranging the calculations in the following sequence, the dry yield (absolute volume of cement, fine aggregate, and coarse aggregate) and yield with entrained air may be determined for use in later calculations. The absolute volumes are determined for a one-sack batch.

Absolute volumes (cu ft) per sack of cement:

Absolute volume	=	(0.85) (0.4782)							
cement									
	=	0.4065 cu ft/sack							
Absolute volume fly ash	=	(0.15) (0.5538)							
ily asii	=	0.0831 cu ft/sack							
Absolute volume	=	(Mix prop) x (Abs vol of 1 cu ft dry rodded fine aggr)							
fine aggregate	=	$(1.70) \times (0.6816) = 1.1587 \text{ cu ft/sack}$							
Absolute volume	=	(Mix prop) x (Abs vol of 1 cu ft dry rodded coarse aggr)							
coarse aggregate	=	$(3.25) \times (0.5872) = 1.9084 \text{ cu ft/sack}$							
Dry yield of	=	Total dry abs vol of cement, fly ash, fine aggr, and coarse aggr							
one-sack batch	=	(0.4065) + (0.0831) + (1.1587) + (1.9084)							
	=	3.5567 cu ft/sack							
Absolute volume of water	=	4.9 gal/sack 7.5 gal/cu ft							
	=	0.6533 cu ft/sack							
Yield without	=	(Dry yield) + (Absolute volume of water)							
entrained air	=	(3.5567) + (0.6533)							
	=	4.2100 cu ft/sack							
Yield with entrained air	=	Yield without entrained air (100 - Percent air) 100							
	=	4.2100 (100 - 5.5) 100							
	=	4.2100 0.945							
	=	4.4550 cu ft/sack							



Absolute volume = (Yield with entrained air) - (Yield without entrained air) of entrained air

= 4.4550 - 4.2100

= 0.2450 cu ft/sack

The cement factor is determined by dividing 27.0 cu ft (1 cu.yd.) by the absolute volume of concrete produced (yield with entrained air) from a onesack mix.

Cement factor = 27.0 cu ft/cu yd Yield with entrained air of onesack batch

= $\frac{27.0}{4.4550}$

= 6.06 sacks/cu yd

501.2.3.2 Percent excess mortar. Excess mortar is the amount of mortar contained in the mixturethat is not needed to fill the voids in the coarse aggregate. These voids can be visualized as the unfilled space in a container of dry-rodded coarse aggregate. Mortar is that part of the mixture consisting of fine aggregate, cement, fly ash, water, and air. To calculate the percent excess mortar, the voids in the coarse aggregate and the absolute volume of mortar must be calculated. The calculations are based on a one-sack batch.

Voids in dry-rodded coarse aggregate, the design mix proportion)
- (Absolute volume of coarse aggregate in a one-sack batch)

= 3.25 - (3.25 x 0.5872)

= 3.25 - 1.9084

= 1.3416 cu ft/sack

Absolute volume = Sum of abs vol of fine aggr, cement, fly ash, water, and air of mortar (See calculations for factor)

 $\hspace{35pt} = \hspace{35pt} 1.1587 + 0.4065 + 0.0831 + 0.6533 + 0.2450$

= 2.5466 cu ft/sack

Percent excess = [Abs vol of mortar - Voids in dry-rod coarse aggr] x 100 Mortar Voids in dry-rodded coarse aggregate

> = (2.5466 - 1.3416) x 100 1.3416

= 89.82%, rounds to 90%

501.2.3.3 Percent fine aggregate. If Method A is used for mix design, it is beneficial, for purposes of verifying the design, to calculate the percent fine aggregate by absolute volume.



x 100

$$\frac{1.1587}{1.1587 + 1.9084}$$

501.2.3.4 Dry weights per cubic yard batch. All calculations to this point have been based on a onesack batch. To determine the amounts of each ingredient, on a dry basis, contained in a cubic yard batch multiply the mix proportion for that ingredient by the number of sacks of cement (cement factor) per cubic yard by the unit weight of that ingredient. Round the results to the nearest pound.

Dry weight per = (Mix proportion) x (Cement factor) x (Dry rodded unit weight)

cubic yard batch

Cement = $1 \times (0.85) \times (6.06 \text{ sacks/cu yd}) \times (94 \text{ lb/sack})$

= 486 lb/cu. yd.

Fly Ash = $1 \times (0.15) \times (6.06 \text{ sacks/cu.yd.}) \times (94 \text{ lb./sack})$

= 86 lb/cu. yd.

Fine aggregate = 1.70 x (6.08 sacks/cu yd) x (111 lb/cu ft)

= 1147 lb/cu yd

Coarse aggregate = 3.25 x (6.08 sacks/cu yd) x (96 lb/cu ft)

= 1897 lb/cu yd

501.2.3.5 Yield per cubic yard batch. The dry yield and yield with entrained air for a onesack batch were determined as an intermediate step in calculating the cement factor in paragraph 501.2.3.1 of this Section. To determine dry yield for a one cubic yard batch, multiply the dry yield for a one-sack batch by the cement factor, in sacks.

Dry yield = 3.5567×6.06

= 21.62 cu ft per cubic yard

To determine total yield for a one cubic yard batch, multiply the yield with entrained air for a onesack batch by the cement factor, in sacks.

Total yield = 4.4550×6.06

= 27.09 cu ft

501.2.3.6 Percent Air in Mortar. Air entrainment is generally considered effective for freeze-thaw resistance when the volume of air in the mortar fraction of the concrete (material passing the No. 4 sieve) is about 9%± 1%.



The following procedure is to be used to compute this value.

```
Percent Air in Mortar = \frac{\text{(Abs. vol. of entrained air)} \times 100}{\text{(Abs. vol. total conc. in one-sack batch - Abs. vol. coarse agg.)}}
= \frac{(0.2450) \times 100}{(4.4550) - (1.9084)}
= 9.6207
```

Rounded to the nearest 0.1%, the Percent Air inMortar for this mix is 9.6%.

Generally, this value will not fluctuate outside of the 8 to 10 percent range when using an assumed air content of 5.5% and the percentages of fine aggregate as suggested in Section 501.2.2. The amount of material in the coase aggregate passing the No. 4 sieve is considered to be negligible for the purpose of this design check.

- **501.2.4 Field Proportions.** The proportion of the various materials to be incorporated into the mix, the effective water, the scale weights, yield, and cement factor may be determined from the equations and procedures described in Sec 501.6 of the Construction Manual.
- **501.3 REPORT.** Data collected at the concrete plant, or related to the day's production, shall be documented in SiteManager as sample <code>ecords</code>. A concrete plant inspector's daily report may be printed using Impromptu (Concrete Plant Inspectors Report) and distributed as described in Sec 500 of the Construction Manual.
- **501.4 PRECAST CONCRETE.** The use of approved high range waterreducers and non-chloride accelerators will be permitted for use in precast concrete having a minimum cement content of 564 pounds and no other specific air or slump mix requirements, if approved by the district Operations Engineer except for concrete pipe. Requests for additives to concrete pipe should be directed to the State Materials Engineer.
- **501.4.1** The producer shall furnish a mix design to the district Operations Engineer, for approval, which shall indicate the batch weights, including the amount of admixure(s) to be used.
- **501.4.2** The admixture(s) are to be within the manufacturer's recommended dosage levels.
- **501.4.3** The accelerators shall be of the non-chloride type.
- **501.4.4** The supplier of the high range waterreducer shall furnish for the district Gerations Engineer's file, a certification stating the admixture being furnished is considered to be compatible with the other admixtures (i.e., air entrainment, retarder, accelerator, etc. by brand name) being used in the mixture. Any changes in sourcesof admixtures will require a re-submittal of the certification.
- **501.4.5** A copy of the approval of the mix design using the above admixtures shall be submitted to the State Materials Engineer.
- **501.5 CLASS A-1 CONCRETE.** Class A-1 Concrete was created for prestress work. It provides a smaller aggregate, which is necessary to accommodate the often minimal clearance between forms and reinforcement, or within the reinforcement cage. It also provides additional cement to accommodate the water demand associated with the smaller rock, and to obtain faster strength gain. Class Al Concrete for work other than prestress may be modified as follows.
- **501.5.1** Grade D stone may be substituted for Grade E stone, when the piece being cast does not have clearance



problems that might cause Grade D stone to bridge and cause voids or low density areas within the piece. When the District and the precaster are in agreement on the desirability of the Grade D substitution, Headquarters Materials shall be contacted with the proposal.

- **501.5.2** Headquarters Materials will review the proposal and will approve or disapprove the substitution of Grade D stone for Grade E stone in the designated application.
- **501.5.3** Following approval by Headquarters Materials, a no-cost change order shall be issued on the associated contract to record the intent to use a nonspecified substitution of material.



TABLE 1 FIELD SECTION 501

Absolute Volumes Per Cubic Foot of Material

	ecific wity 80	81		_				n poun 86		88	89	90	91	92	93	94
2.36	.5432 .550	0 55	68 4	5636	5704	5772	5840	5908	5976	5 6044	4 611	1 6179	6247	6315	6383	3
2.37	.5409 .547															
2.38	.5387 .545															
2.39	.5364 .546	1 .54	98 .5	5565	.5632	.5699	.5767	7 .5834	.5901	.5968	.603	5 .6102	.6169	6236 .	6303	
2.40	.5342 .540	9 .54	75 .5	5542	.5609	.5676	.5743	.5809	.5876	5 .5943	.601	0 .6076	.6143	.6210	.6277	7
2.41.	.5320 .538	6 54	52 5	5510	5596	5650	5710	5795	5050	5019	500	5 6051	6119	6194	6251	
2.41.	.5298 .536															
2.43	.5276 .534)
2.44	.5254 .532															
2.45	.5233 .529	8 .53	64 .5	5429	.5495	.5560	.5625	.5691	.5756	5 .5822	2 .588	7 .5952	.6018	.6083	.6149)
2.46	5010 505	II 52	12 6	5 407	5 472	5527	5600	5668	5722	2 5700	500	2 5020	5002	(050	6124	
2.46 2.47	.5212 .527 .5190 .525															ŀ
2.48	.5170 .523															L
2.49	.5149 .521															
2.50	.5128 .519															
2.51	.5108 .517															2
2.52 2.53	.5088 .515 .5067 .513															
2.54	.5047 .513															
2.55	.5028 .509															
2.56	.5008 .507															Ļ
2.57	.4989 .505															
2.58	.4969 .503															
2.59 2.60	.4950 .501 .4931 .499															
2.00	.4731 .473	,5 .50	43	3110.	3176	.3239	.5501	.5502	.3424	.5460	.5547	.5009	.5071	.3132	.3794	
2.61	.4912 .497	3 .50	35 .5	5096	.5158	.5219	.5280	.5342	.5403	.5465	5 .562	6 .5587	.5649	.5710	.5772	2
2.62	.4893 .495															
2.63	.4875 .493															
2.64	.4856 .491)
2.65	.4838 .489	<i>1</i> 8 .49	39	3019	.5080	.51₩	.5201	.5201	.3322	.3382	.5443	.5505	.3304	.3024	.3083	
2.66	.4820 .488	30 .49	40 .5	5000	.5061	.5121	.5181	.5241	.5302	2 .5362	2 .542	2 .5482	2 .5543	.5603	.5663	3
2.67	.4802 .486	52 .49	22 .4	4982	.5042	.5102	.5162	2 .5222	.5282	2 .534	2 .540	2 .5462	2 .5522	2 .5582	2 .564	
2.68	.4784 .484															
2.69	.4766 .482)
2.70	.4748 .480)8 .48	67.4	4926	.4986	.5045	.5104	4.564	.5223	.5283	.5342	.5401	.5461	.5520	.5579	
2.71	.4731 .479	0 .48	49 .4	4908	.4967	.5026	.5086	5 .5145	.5204	1.5263	3 .532	2 .5381	.5440	.5500	.5559)
2.72	.4713 .477															
2.73	.4696 .475															
2.74	.4679 .473															3
2.75	.4662 .472	20 .47	79 .4	4837	.4895	.4953	.5012	2 .5070	.5128	8 .586	.5245	.5303	.5361	.5420 .	5478	
2.76	.4645 .470	3 47	61 /	1810	4877	4035	4003	3 5050	5110) 5169	3 522	6 5284	L 53/12	5400	5/158	2
2.77	.4628 .468															
2.78	.4612 .466															
2.79	.4595 .465	3 .47	10 .4	4767	.4825	.4882	.4940	.4997	.5055	.5112	2 .517	0.5227	.5284	.5342	.5399	
2.80	.4579 .463	36 .46	93 .4	4750	.4808	.4865	.4922	2 .4979	.503	7 .509	4 .515	15208	.5266	.5323 .	5380	
2.81	.4562 .461	0 16	77 /	1721	4701	1010	4004	1060	5010	5074	5 512	3 5100	5247	5204	5261	
2.81	.4546 .460															
2.83	.4530 .458															•
2.84	.4514 .457															Ļ
2.85	.4498 .455															



TABLE 1 (Cont'd) FIELD SECTION 501

Absolute Volumes Per Cubic Foot of Material

Specific	1 1	.DBO	V			bic foo				oot	OI IV	ıuı) I I I I I	•	
Gravity		96	97	98	99	100	101		103	104	105	016	107	108	109
2.36											.7130				
											.7100				
											.7070 .				
											.7401				
2.40	.6343	.6410	.5477	.6544	.6611	.6677	.5744	.6811	.6878	.6944	.7011	.7078	.7154.	7212 .7	1278
2.41	6217	6201	6150	6517	6502	6502	6650	6716	6702	6940	.6916	6002	7040	7115	7102
2.41											.6953				
											.6825 .				
											.6896				
											.6868				
2.13	.0211	.02//	.05 15	.0110	.0170	.05 11	.0000	.0072	.0757	.0005	.0000	.0751	.0///	.7001	. 1240
2.46	.6189	.6254	.6319	.6384	.6449	.6514	.6580	.6645	.6710	.6775	.6840	.6905	.6971	.7036	.7101
											.6813				
											.6785 .				
											.6758				
2.50	.6090	.6154	.6218	.6282	.6346	.6410	.6474	.6538	.6603	.6667	.6731	.6795	.6859	.6923	.6987
2.51	6065	6120	6102	6257	6321	6395	6/10	6512	6576	6640	.6704	6769	6822	6805	6050
											.6677				
											.6651 .				
											.6625				
											.6599				
											.6573				
											.6547				
											.652 .0				
											.6497				
2.60	.5856	.5917	.5979	.6040	.6102	.6164	.6225	.6287	.6349	.6410	.6472	.6534	.6595	.6657	.6718
2.61	5833	5894	5956	6017	6079	6140	6201	6263	6324	6386	.6447 .	6508	6570	6631	6693
											.6422				
											.6398				
2.64	.5767	.5828	.5888	.5949	.6010	.6070	.6131	.6192	.6252	.6313	.6374	.6435	.6495	.6556	.6617
											.6350				
											.6326 .				
											.6302				
											.6279 .6255				
											.6233				
2.70	.3039	.5096	.5151	.3617	.5670	.3933	.3993	.0054	.0113	.0173	.0232	.0292	.0331	.0410	.0470
2.71	.5618	.5677	.5736	.5795	.5854	.5914	.5973.	6032 .	6091 .	6150	.6209 .	6268 .	6327	.6387 .	6446
2.72	.5597	.5656	.5715	.5774	.5833	.5892	.5951	.6010	.6069	.6127	.6186	.6245	.6304	.6363	.6422
2.73	.5577	.5636	.5694	.5753	.5811	.5870	.5929	.5988	.6046	.6105	.6164	.6222	.6281	.6340	.6399
2.74	.5556	.5615	.5673	.5732	.5790	.5849	.5907	.6966	.6024	.6083	.6141	.6200	.6258	.6317	.6375
2.75	.5536	.5594	.5653	.5711	.5769	.5828	.5886	.5944	.6002	.6061	.6119	.6177	.6235	.6294	.6352
2.76	5516	5571	5622	5600	5710	5006	5061	5022	5001	6020	6007	<i>C</i> 155	6212	6071	6220
											. 6097. .6075				
											.6053				
											.6031				
											.6010				
											.598 .6				
											.5967				
											.5946				
											.5925 .				
2.85	.3342	.3398	.5454	.5511	.550/	.3023	.50/9	.5133	.5192	.3048	.5904	.5900	.001/	.00/3	.0129



TABLE 1 (Cont'd) FIELD SECTION 501 Absolute Volumes Per Cubic Foot of Material

Weight per cubic foot in pounds Specific Gravity 110 111 112 113 114 115 116 117 118 119 120 121 122 123 124 2.36 .7470 .7537 .7602 .7673 .7741 .7809 .7877 .7945 .8013 .8081 .8149 .8217 .8284 .832 .8420 .7438 .7506 .7573 .7641 .7709 .7776 .7844 .7911 .7979 .8047 .8114 .8182 .8249 .8317 .8385 .7407 .7474 .7541 .7609 .7676 .7743 .7811 .7878 .7945 .8013 .8080 .8147 .8215 .8282 .8349 .7376 .7443 .7510 .7577 .7644 .7711 .**7**78 .7845 .7912 .7979 .8046 .8113 .8180 .8248 .8315 .7345 .7412 .7479 .7545 .7612 .7679 .7746 .7812 .7879 .7946 .8013 .8080 .8146 .8213 .8280 2.41 .7315 .7381 .7448 .7514 .7581 .7647 .7714 .7780 .7847 .7913 .7980 .8046 .8113 .8179 .8246 2.42 .7284 .7351 .7417 .7483 .7549 .7615 .7682 .7748 .7814 .7880 .7947 .8013 .8079 .8145 .8211 $2.43 \quad .7254 \; .7320 \; .7386 \; .7452 \; .7518 \; .7584 \; .7950 \; .7716 \; .7782 \; .7848 \; .7914 \; .7980 \; .8046 \; .8112 \; .8178$ 2.44 .7225 .7290 .7356 .7422 .7487 .7553 .7619 .76847.750 .7816 .7881 .7947 .8013 .8078 .8144 2.45 .7195 .7261 .7326 .7391 .7457 .7522 .7588 .7653 .7718 .7784 .7849 .7915 .7980 .8046 .8111 .7166 .7231 .7296 .7361 .7427 .7492 .7557 .7622 .7687 .7752 .7817 .7883 .7948 .8013 .8078 .7137 .8202 .7267 .7332 .7396 .7461 .7526 .7591 .7656 .7721 .7786 .7851 .7915 .7980 .8045 2.47 2.48 .7108 .7173 .7237 .7302 .7367 .7431 .7496 .7560 .7625 .7690 .7754 .7819 .7884 .7948 .8013 2.49 $.7080\ .7144\ .7208\ .7273\ .7337\ \ .7401\ .7466\ .7530\ .7594\ .7659\ .7723\ .7788\ .7852\ .7916\ .7981$.7051 .7115 .7179 .7244 .7308 .7372 .7436 .7500 .7564 .7628 .7692 .7756 .7821 .7885 .7949 2.50 2.51 .7023 .7087 .7151 .7215 .7279 .7342 .7406 .7470 .7534 .7598 .7662 .7726 .7789 .7853 .7912 2.52 .6995 .7059 .7123 .7186 .7250 .7313 .7377 .7440 .7504 .7568 .7631 .7695 .7758 .7822 .7886 $2.53 \quad .6968 \ .7031 \ .7094 \ .7158 \ .7221 \ .7284 \ .7348 \ .7411 \ .7474 \ .7538 \ .7601 \ .7664 \ .7728 \ .7791 \ .7854$ $2.54 \quad .6940 \ .7003 \ .7066 \ .7130 \ .7193 \ \ .7256 \ .7319 \ .7382 \ .7445 \ .7508 \ \ .7571 \ .76 \ .769 \ .7760 \ .7824$.6913 .6976 .7039 .7102 .7164 .7227 .7290 .7353 .7416 .7479 .7541 .7604 .7667 .7730 .7793 2.56 .6886 .6949 .7011 .7074 .7136 .7199 .7262 .7324 .7387 .7449 .7512 .7575 .7637 .7700 .7762 $2.57 \quad .6859 \ .6922 \ .6984 \ .7046 \ .709 \ \ .7171 \ .7233 \ .7296 \ .7358 \ .7420 \ \ .7483 \ .7545 \ .7608 \ .7670 \ .7732$ 2.58 .6833 .6895 .6957 .7019 .7081 .7143 .7205 .7267 .7330 .7392 .7454 .7516 .7578 .7640 .7702 2.59 .6806 .6868 .6930 .6992 .7054 .7116 .7178 .7239 .7301 .7363 .7425 .7487 .7549 .716 .7673 2.60 .6780 .6842 .6903 .6965 .7027 .7088 .7150 .7212 .7273 .7335 .7396 .7458 .7520 .7581 .7643 2.61 .6754 .6816 .6877 .6938 .7000 .7061 .7123 .7184 .7245 .7307 .7368 .7430 .7491 .7552 .7614 2.62 .6728 .6789 .6851 .6912 .6973 .7034 7095 .7156 .7218 .7279 .7340 .7401 .7462 .7523 .7585 2.63 .6703 .6764 .6825 .6886 .6946 .7007 .7068 .7129 .7190 .7251 .7312 .7373 .7434 .7495 .7556 2.64 .6677 .6738 .6799 .6859 .6920 .6981 .7042 .7102 .7163 .7224 .7284 .7345 .7406 .7466 .7527 2.65 .6652 .6713 .6773 .6834 .6894 .6955 .7015 .7075 .7136 .7196 .7257 .7317 .7378 .7438 .7499 2.66 .6627 .6687 .6748 .6808 .6868 .6928 .6989 .7049 .7109 .7169 .7230 .7290 .7350 .7410 .7471 2.67 .6602 .6662 .6722 .6782 .6842 .6902 .6962 .70227052 .7143 .7203 .7263 .7323 .7383 .7443 $2.68 \quad .6578 \; .6637 \; .6697 \; .6757 \; .6817 \; .6877 \; .6936 \; .6996 \; .7056 \; .7116 \; .7176 \; .7235 \; .7295 \; .7355 \; .7415$.6553 .6613 .6672 .6732 .6792 .6851 .6911 .6970 .7030 .7089 .7149 .7209 .7268 .7328 .7387 2.69 2.70 .6529 .6588 .6648 .6707 .6766 .6826 .6885 .6944 .7004 .7063 .7123 .7182 .7241 .7301 .7360 $2.71 \quad .6505 \; .6564 \; .6623 \; .6682 \; .6741 \; .6801 \; .6860 \; .6919 \; .6978 \; .7037 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096 \; .7155 \; .7214 \; .7274 \; .7333 \; .7096$ 2.72 .6481 .6540 .6599 .6658 .6717 .6776 .6834 .6893 .6952 .7011.7070 .7129 .7188 .7247 .7306 2.73 .6457 .6516 .6575 .6633 .6692 .6751 .6809 .6868 .6927 .6986 .7044 .7103 .7162 .7220 .7279 2.74 .6434 .6492 .6551 .6609 .6668 .6726 .6785 .6843 .6902 .6960 .7019 .7077 .7136 .7194 .7252 .6410 .6469 .6527 .6585 .6643 .6702 .6760 .6818 .6876 .6935 .6993 .7051 .7110 .7168 .7226 2.76 .6387 .6445 .6503 .6561 .6619 .6677 .6735 .6793 .6852 .6910 .6968 .7026 .7084 .7142 .7200 2.77 .6364 .6422 .6480 .6538 .6595 .6653 .6711 .6769 .6827 .6885 .6943 .700 .7058 .7116 .7174 .6341 .6399 .6456 .6514 .6572 .6629 .6687 .6745 .6802 .6860 .6918 .6975 .7033 .7090 .7148 .6318 .6376 .6433 .6491 .6548 .6606 .6663 .6720 .6778 .6835 .6893 .6950 .7008 .7065 .7123 2.80 .6296 .6353 .6410 .6467 .5**2**5 .6582 .6639 .6696 .6754 .6811 .6868 .6925 .6983 .7040 .7097 2.81 .6273 .6330 .6387 .6444 .6502 .6559 .6616 .6673 .6730 .6787 .6844 .6904 .6958 .7015 .7072 2.82 .6251 .6308 .6365 .6422 .6478 .6535 .6592 .6649 .6706 .6763 .6819 .6876 .6933 990 .7047 .6229 .6286 .6342 .6399 .6456 .6512 .6569 .6625 .6682 .6739 .6795 .6852 .6909 .6965 .7022 .6207 .6264 .6320 .6376 .6433 .6489 .6546 .6602 .6659 .6715 .6771 .6828 .6884 .6941 .6997 .6185 .6242 .6298 .6354 .6410 .6466 6523 .6579 .6635 .6691 .6748 .6804 .6860 .6916 .6973



TABLE 1 (Cont'd) FIELD SECTION 501 Absolute Volumes Per Pound of Material

	Mosorate	volumes i ci i	dulla of Materi
	Absolute		Absolute
Specific	Volume in	Specific	Volume in
Gravity	Cu. Ft.	Gravity	Cu. Ft.
2.36	0.006790526	2.61	0.006140092
2.37	0.006761874	2.62	0.006116657
2.38	0.006733463	2.63	0.006093400
2.39	0.006705289	2.64	0.006070319
2.40	0.006677350	2.65	0.006047412
2.41	0.006649644	2.66	0.006024677
2.42	0.006622166	2.67	0.006002113
2.43	0.006594914	2.68	0.005979717
2.44	0.006567886	2.69	0.005957487
2.45	0.006541078	2.70	0.005935423
2.46	0.006514488	2.71	0.005913521
2.47	0.006488114	2.72	0.005891780
2.48	0.006461952	2.73	0.005870198
2.49	0.006436000	2.74	0.005848774
2.50	0.006410256	2.75	0.005827506
2.51	0.006384718	2.76	0.005806392
2.52	0.006359381	2.77	0.005785430
2.53	0.006334245	2.78	0.005764619
2.54	0.006309308	2.79	0.005743957
2.55	0.006284565	2.80	0.005723443
2.56	0.006260016	2.81	0.005703075
2.57	0.006235658	2.82	0.005682851
2.58	0.006211489	2.83	0.005662771
2.59	0.006187506	2.84	0.005642831
2.60	0.006163708	2.85	0.005623032
		3.15	0.005087505



Inter-Office Correspondence

MISSOURI DEPARTMENT OF TRANSPORTATION

DATE:	February 13, 1991		
TO:			
FROM:	District Operation	s Enginær	
SUBJECT:	Materials Portland Cement (Concrete Mix Propor	tions
	Wt./Cu. Ft.	Sp. Gr.	% Absorption
Sand Stone Fly As	h		

		Water/				Dry	DRY WEIGHT - (27.0 CU.FT.)					
		Cement				Yield			_		~	
		Ratio		% Air		for	CEM	ENT	Fly	Fine	Coarse	
Class of	Mix		%	in	Cement	27.0			Ash	Agg	Agg	
Concrete	Proportions	Gal./Sk	Air	Mortar	Factor	Cu.Ft	Lbs.	Sacks	Lbs.	Lbs.	Lgs.	

Exhibit 501-A Portland Cement Concrete Mix Proportions

